

CADASTRAL DATABASE POSITIONAL ACCURACY IMPROVEMENT

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ABSTRACT:

Positional Accuracy Improvement (PAI) is the refining process of the geometry feature in a geospatial dataset to improve its actual position. This actual position relates to the absolute position in specific coordinate system and the relation to the neighborhood features. With the growth of spatial based technology especially Geographical Information System (GIS) and Global Navigation Satellite System (GNSS), the PAI campaign is inevitable especially to the legacy cadastral database. Integration of legacy dataset and higher accuracy dataset like GNSS observation is a potential solution for improving the legacy dataset. However, by merely integrating both datasets will lead to a distortion of the relative geometry. The improved dataset should be further treated to minimize inherent errors and fitting to the new accurate dataset. The main focus of this study is to describe a method of angular based Least Square Adjustment (LSA) for PAI process of legacy dataset. The existing high accuracy dataset known as National Digital Cadastral Database (NDCDB) is then used as bench mark to validate the results. It was found that the propose technique is highly possible for positional accuracy improvement of legacy spatial datasets.

1. INTRODUCTION

Many countries around the world have recognized and appreciated the value of accurate digital cadastral database. Theoretically, an accurate, efficient and updated cadastral database offers the better basis for planning and implementation of variety of real estate application (Durdin, 1993; Effenberg et al. 1999; Ting et al., 1999). The advancement in spatial based technology like Geographic Information System (GIS) also suggested an urgent need to maintain the spatial data in high accuracy to represent the real world.

Principally, numerous spatial datasets were previously digitized from hardcopy maps. The legacy datasets in use today are a combination of data from different sources, of technologies and measurement techniques. As a result, these legacy datasets have relatively low positional accuracy caused by errors resulting from the production and measurement method employed according to the technological and legal changes over time (Sisman, 2014). In addition, the common error in digitizing process such as distortion of source map, digitizing operational errors and ground coordinate system constituted a combination of systematic and random errors (Tong, Shi, & Liu, 2009). However, the urgency of updating legacy dataset is extremely crucial, the needs for combine spatial data from different sources and accuracy has dramatically increased. This process is crucial to allow different datasets to be jointly presented and analyzed. The process requires an understanding about the positional accuracies of the geometries in the datasets to avoid mismatches and misinterpretations of geospatial data.

Generally, resurvey, reprocessing the existing survey data and upgrading the existing cadastral datasets are the potential

approaches for improving the legacy cadastral datasets. Resurvey all the cadastral parcel probably is the best technique to generate the new accurate dataset, however this process require lengthy time and estimated cost is very high (Arvanitis et al., 1999). Meanwhile, Buyong et al. (1992) and Durdin (1993) suggested that maintaining the old measurement and incorporating the new measurements will suffice for upgrading legacy datasets. However, Perelmutter et al. (1992) stated that the incorporating is unsuitable due to weakness of the original control network (datum) and field book record system. Additionally, from the economic perspective, 20000 existing field books for instance, will require hundreds of operators and take many years to accomplish (Fradkin et al., 2002).

A possible alternative for improving the legacy datasets with realistic cost is transforming the legacy dataset into new system. This high possibility supported by the advancement of satellite base technology especially Global Navigation Satellite System (GNSS). Possibly, the coordinate accuracy of the legacy dataset can be highly improved. Furthermore, the availability of high resolution aerial imagery offer high possibilities of using imagery as background when underlays to spatial dataset (Hope et al., 2008). The imagery which has high quality spatial resolution and absolute accuracy like Quick Bird can be used as a base map to check the relative position of the adjusted legacy datasets.

Essentially, the legacy dataset accuracy has to be improved line with the current high accuracy spatial technology. As a result, many organizations and researchers around the world used PAI approach for upgrading legacy datasets into high accuracy dataset (Donnelly et al.2006; Fradkin et al., 2002; Hesse et al., 1990; Morgenstren, 1989; Tamim, 1995). Based on the

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importance of PAI, the focus of this paper is to propose the PAI method using angular based Least Square Adjustment (LSA). The detail discussion on PAI is explained in the following section.

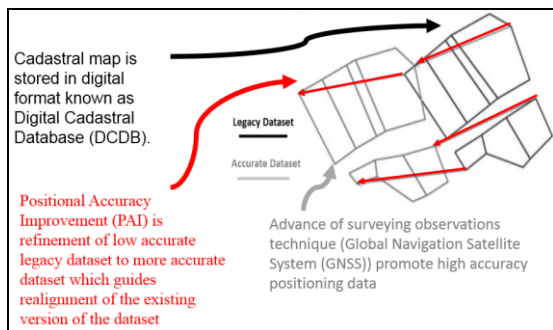


Figure 1. PAI concept (Hashim et al, 2016)

2. POSITIONAL ACCURACY IMPROVEMENT

PAI is a process of improving the position of the geometric coordinates of a feature in a geospatial dataset to represent its actual position (Rönsdorf, 2008). The PAI can be classified as the improvement of low accurate legacy dataset to more accurate dataset. Figure 1 illustrates the concept of PAI (Norshahrizan et al, 2016).

PAI is commonly applied in two situations, PAI of Reference Data and PAI of User Data (Rönsdorf, 2008). The PAI of Reference Data links with improving the position of geometries in a reference dataset that describes physical or abstract features of the earth. The features position relate to the absolute position in a standard Coordinate Reference System such as Geocentric Datum of Malaysia 2000 (GDM 2000) in case of Malaysia or WGS-84 in global coordinate system. Meanwhile, the PAI of User Data describes the successive synchronization of legacy datasets with the already positionally improved reference dataset in order to retain the relationships between geometries.

To achieve an optimal solution in PAI, the method of LSA is often employed towards improving the positional accuracy of spatial datasets (Tamim 1995, Wolf and Ghilani 2006, Merritt and Masters 1999, Gielsdorf et al. 2004, Merritt 2005, Tong et al. 2005, Casado 2006, Hope et al. 2008, Tong et al. 2009). The LSA method is a well-established technique for solving an overdetermined system of equations by minimizing a weighted quadratic form of the residuals. Its application in estimating parameters in coordinate transformation can be found in literature; for example, Mikhail and Ackemann (1982), Wolf and Ghilani (2006), and Koch (1999). Tamim (1995) presented a methodology to create a digital cadastral overlay through upgrading digitized cadastral data. Merritt and Masters (1999) and Merritt (2005) developed the spatial adjustment engine based on the least squares method and applied it to improve the accuracy of cadastral data in Australia. Tong et al. (2005) presented a least squares adjustment model to resolve inconsistencies between the digitized and registered areas of cadastral parcels, and further improved the adjustment model by introducing scale parameters to reduce the influence of systematic error in the adjustment (Tong et al. 2009). Felus (2007) presented a workflow of three steps used to enhance the spatial accuracy of digital cadastral maps: a global

transformation from an old local system to a GPS-based WGS-84 system; a rubber-sheeting transformation for modifying boundary corners to fit existing ground features; and a LS adjustment with stochastic constraints to include additional cadastral information and geometric conditions. Hope et al. (2008) proposed a method of least squares with inequalities for data integration, in which topological relationships are modeled in the form of inequalities and optimal positioning solutions are obtained while preserving the spatial relationships among features.

Since the legacy dataset are less accurate in positioning, the integration of legacy datasets and highly accurate data such as those from GNSS is one of the most possible methods to improve the legacy datasets accuracy (Hope et al., 2008). However, it was found from Hope et al. (2008) that by simply replacing a sample of legacy dataset with more accurate version will lead to a distortion of the neighboring geometry (Figure 2). In addition, the relative geometry of the legacy datasets is often better than its absolute accuracy and supposed to be the spatial relationship or relative geometry between features must be preserved.

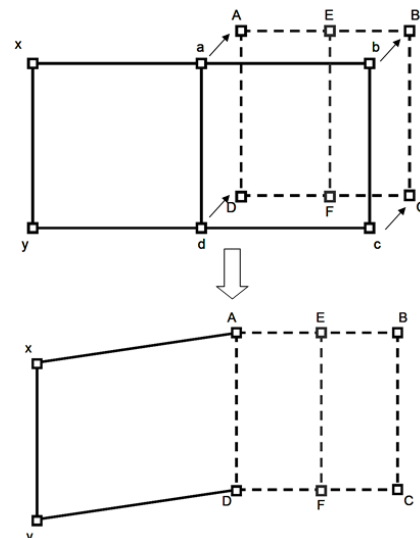


Figure 2. Cadastral boundaries (Solid Lines) and higher accuracy (Dashed Lines). Polygon xady is distorted if point abcd are simply replaced by ABCD (Hope et al., 2008)

The previous studies discussed above largely used the additional new observations which link with the legacy data for maintaining the geometry of adjusted data. In this study, the existing data alone from certified plan is used for maintaining the geometry of legacy data in the adjustment stage. The following section describes how the distance, bearings and angles are used in the LSA process.

2.1 Least square adjustment (LSA)

LSA is a model of solution widely used in the disciplines of surveying. The conventional Bowditch method is known as an arbitrary method since the corrections to the observations are applied irrespective of their uncertainties, whereas the least square adjustment method is more advanced technique. It adjusts observations based on the laws of probability, which models the occurrence of random errors (Halim et al., 2001). In other words, the LSA principle is such that the squares of the

residuals must be minimized. Equation (1) expresses the fundamental principle of least squares:

$$\sum v^2 = v_1^2 + v_2^2 + \dots + v_n^2 = \text{minimum} \quad (1)$$

where v = residual

There are two mathematical models in LSA, stochastic and functional model. The stochastic model involves the determination of variances and the weights of the observations. A functional model is an equation or set of equations that represents or defines an adjustment condition. There are two types of functional models, the conditional and parametric adjustments. In a conditional adjustment, geometric conditions are compulsorily imposed on the measurement and their residuals, for instance, the sum of the angles in a closed polygon is $(n-2) \times 180$. In the parametric adjustment, observations are expressed in terms of unknown parameters that were never observed directly. For example, the well-known coordinate equations are used to model the angles, directions, and distances observed in a horizontal plane survey. The adjustment yields the most probable values for the coordinates (parameters), which in turn provide the most probable values for the adjusted observations.

In this study, the parametric adjustment of functional model is applied. The three mathematical models of observations, horizontal angle, azimuth and distance were used in the adjustment process. The mathematical model of bearing, horizontal angle and distance observation are like equations (2), (3) and (4) respectively (Amat, 2007; Setan, 1995; Wolf and Ghilani (2006).

$$\theta_1 = \tan^{-1} \left(\frac{X_B - X_A}{Y_B - Y_A} \right) \quad (2)$$

$$\theta_1 = F(X_{AO}, Y_{AO}, X_{BO}, Y_{BO}) + \left(\frac{Y_{BO} - Y_{AO}}{L_{AB}^2} \right) \cdot \partial X_A + \left(\frac{Y_{AO} - Y_{BO}}{L_{AB}^2} \right) \cdot \partial X_B + \left(\frac{X_{BO} - X_{AO}}{L_{AB}^2} \right) \cdot \partial Y_A + \left(\frac{X_{AO} - X_{BO}}{L_{AB}^2} \right) \cdot \partial Y_B$$

where θ_1 = Bearing
 X_{AO}, Y_{AO} = Point A estimated coordinate
 X_{BO}, Y_{BO} = Point B estimated coordinate
 L_{AB} = Estimated horizontal distance A-B

$$\Delta\theta = \tan^{-1} \left(\frac{X_D - X_A}{Y_D - Y_A} \right) - \tan^{-1} \left(\frac{X_B - X_A}{Y_B - Y_A} \right) \quad (3)$$

$$\Delta\theta = F(X_{AO}, Y_{AO}, X_{BO}, Y_{BO}, X_{DO}, Y_{DO}) + \left(\frac{Y_{AO} - Y_{DO}}{L_{AD}^2} + \frac{Y_{BO} - Y_{AO}}{L_{AB}^2} \right) \cdot \partial X_A + \left(\frac{Y_{AO} - Y_{BO}}{L_{AB}^2} \right) \cdot \partial X_B + \left(\frac{Y_{DO} - Y_{AO}}{L_{AD}^2} \right) \cdot \partial X_D + \left(\frac{X_{AO} - X_{DO}}{L_{AD}^2} + \frac{X_{BO} - X_{AO}}{L_{AB}^2} \right) \cdot \partial Y_A + \left(\frac{X_{AO} - X_{BO}}{L_{AB}^2} \right) \cdot \partial Y_B + \left(\frac{X_{DO} - X_{AO}}{L_{AD}^2} \right) \cdot \partial Y_D$$

where $\Delta\theta$ = Horizontal angle

X_{AO}, Y_{AO} = Point A estimated coordinate

X_{BO}, Y_{BO} = Point B estimated coordinate

X_{DO}, Y_{DO} = Point D estimated coordinate

L_{AB} = Estimated horizontal distance A-B

L_{AD} = Estimated horizontal distance A-D

$$L_{AB} = \sqrt{(X_B - X_A)^2 + (Y_B - Y_A)^2} \quad (4)$$

$$L_{AB} = F(X_{AO}, Y_{AO}, X_{BO}, Y_{BO}) + \left(\frac{X_{AO} - X_{BO}}{L_{AB}^2} \right) \cdot \partial X_A + \left(\frac{X_{BO} - X_{AO}}{L_{AB}^2} \right) \cdot \partial X_B + \left(\frac{Y_{AO} - Y_{BO}}{L_{AB}^2} \right) \cdot \partial Y_A + \left(\frac{Y_{BO} - Y_{AO}}{L_{AB}^2} \right) \cdot \partial Y_B$$

where L_{AB} = Distance

X_{AO}, Y_{AO} = Point A estimated coordinate

X_{BO}, Y_{BO} = Point B estimated coordinate

L_{AB} = Estimated horizontal distance A-B

3. METHODOLOGY

The research study area is in Kampung Pasir, Mukim Pulai, Johor Bahru District as shown in Figure 3. The area covers 77 parcels and the details of the data sources are illustrated in Table 1. The raw data for adjustment process was acquired from DSSM Johor in the form of Digital Cadastral Data Base (DCDB) or commonly known as Pangkalan Data Ukur Kadaster (PDUK). In this paper, the existing technique applied by DSMM is declared as DSMM Technique. Meanwhile, the proposed technique to produce the new accurate dataset is known as PAI Technique.

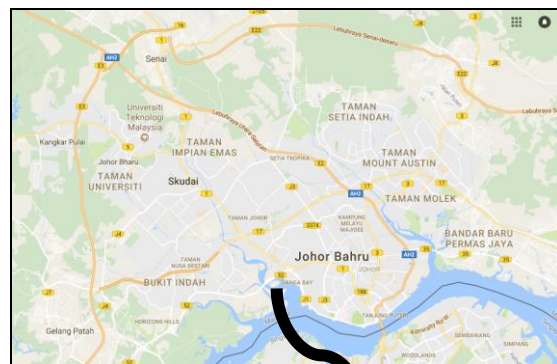


Figure 3. Study area

Table 1. Data sources

Location	Kampung Pasir, Mukim Pulau, Daerah Johor Bahru, Johor, Malaysia
Parcel number	4169-4189, 4199-4225, 119251-119277 & 119279
Certified Plan (CP)	CP 40225 and CP 120293
Data	PDUK (Block boundary line, CRM, Lot)
Data form	Bearing and distance
Data format	.tab
Adjustment Software	Star*net

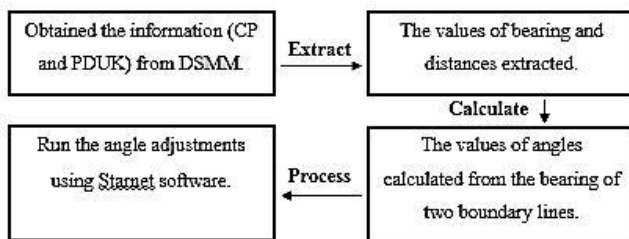


Figure 4. Interior angle extraction

The first step involves the calculation of interior angle of cadastral parcel. The existing raw data in LSA applied by DSMM are bearing and distance. The proposed method in this study is based on the angular and distance based LSA. The main justification of using angle method is to extract the independence or direct observation of raw data which contained minimum gross and systematic error. The bearing value is a very dependent data where it is largely based on the sources of the initial bearing. The common practice of the DSMM in the previous work is based on the solar observation and interior angle of three good boundary marks. As a result, it is extremely difficult to verify the quality of the reference bearing used in the previous work. In this study, the interior angle of these data is considered as an independent data which is calculated from the bearings of two boundary lines. Although the bearing control contains gross and systematic error, the interior angle between two boundary lines is free from the both factors.

The Star*net software has been used for the adjustment purposes. The Star*net is a rigorous least squares analysis software designed to adjust 2D and 3D survey networks. The output consists of a file of adjusted station coordinates and a statistical analysis of the adjustment. In addition, the graphical facilities are provided to allow the user to plot the network, including error ellipses of the adjusted points and relative error ellipses between stations. Star*net has the capacity to weight all input data both independently and by category which means that data can be defined as being FREE or FIXED and anything in between. The Star*net has the ability to control the weighting of input data where Giving accurately known measurements more weight than those measurements known to be less accurate.

The detail step of the processing in Star*net is shown in Figure 5. A new project is created which have two input file, bearing-distance and angle-distance. The bearing-distance input data is the current technique used by DSMM, meanwhile, the angle-distance input data is the new approach which is proposed in this study.

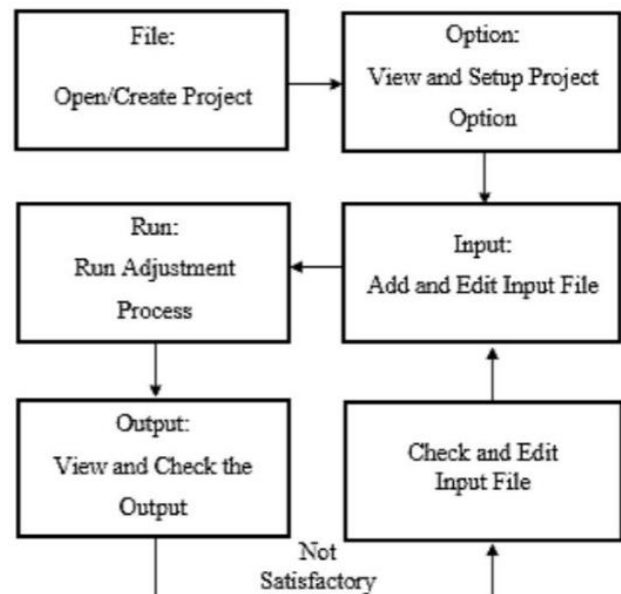


Figure 5. Processing stage in Star*net adjustment software

The final output of the adjustment in Star*net is the adjusted coordinate of entire boundary marks. The final coordinate will be exported to the dxf, coverage and shapefile format for further graphic and spatial based analysis. The AUTOCAD and ARCGIS software are the main tools for the processing. The final output of this process will be submitted in the accurate cadastral database. Figure 6 describes the entire process of suggested PAI as discussed above.

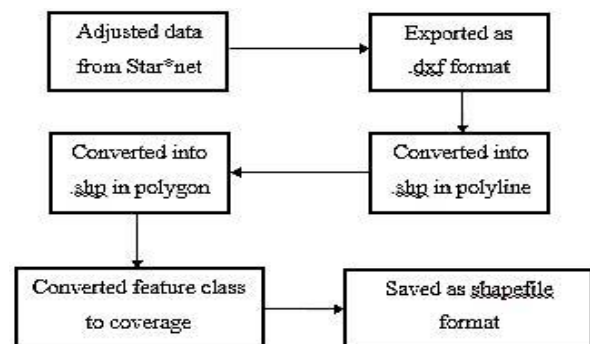


Figure 6. Accurate cadastral database development

4. RESULT AND ANALYSIS

In the verification stages, the output of the PAI Technique was compared to the result of DSMM Technique. The verification is based on four factors, area, bearing, distance and coordinate.

Table 2, 3, and 4 show the sample results of area comparisons by DSMM and PAI techniques. The actual area represents the area extracted from original certified plan (CP) and the adjusted area is obtained from the two different techniques. The results shown are selected from the minimum and maximum area differences of both methods.

Based on the DSMM Technique, the larger area differences given is 1.209 m² while the larger value given by PAI Technique is 0.725 m². In the meantime, the smallest

differences of both techniques are 0.007 m² and 0.003 m² respectively.

Table 2. Minimum area difference

TECHNIQUE	DSMM	PAI
LOT	119258	4222
ACTUAL AREA (m ²)	212	595.9
ADJUSTED AREA (m ²)	211.993	595.897
DIFFERENCE (m ²)	0.007	0.003

Table 3. Maximum area difference

TECHNIQUE	DSMM	PAI
LOT	4189	4189
ACTUAL AREA (m ²)	875.3	875.3
ADJUSTED AREA (m ²)	874.271	874.575
DIFFERENCE (m ²)	1.029	0.725

In the entire data of area comparison analysis, the verification is based on the standard deviation value. The final result indicated that the standard deviation in PAI technique is 0.254 which is smaller than DSMM technique 0.292. The result proved that the PAI Technique is capable of retaining the area which indirectly evident in the geometry or position of relative boundary mark preserved.

Table 4 shows the differences in bearing values from CP, DSMM and PAI. The bearing values of DSMM are the adjusted bearing obtained from bearing adjustment method and bearing values of PAI are the adjusted bearing from angle adjustment method. The values selected represent the smallest, medium and largest bearing differences.

Table 4. Bearing comparison

BM ID		BEARING (dms)			DIFFERENCE (dms)	
FROM	TO	CP	DSMM	PAI	CP-DSMM	CP-PAI
5135308322	5147708142	34-30-40	34-30-40	34-30-20	0-00-00	0-00-20
5267708003	5270207952	26-16-40	26-16-40	26-15-16	0-00-00	0-01-24
5268607586	5280707604	98-17-00	98-16-53	98-18-06	0-00-07	-0-01-06
5314907946	5334808145	134-59-50	135-00-21	135-00-06	-0-00-31	-0-00-16
5360506938	5373807060	132-30-40	132-30-49	132-30-44	-0-00-09	-0-00-04
5514808060	5530008139	117-30-30	117-30-08	117-30-29	0-00-22	0-00-01

The bearing differences were calculated using the original bearings obtained from CP as reference. For DSMM Technique, the smallest differences is 0°0'0" and the larger value given is 0°0'31". The PAI result shows the maximum and minimum value of bearing differences to be 0°1'24" and 0°0'1" respectively. In terms of bearing comparison, 100% of bearing differences were under 30" in DSMM technique, while 88.8% were below 30". The standard deviation of bearing comparison between DSMM and PAI technique are 5.8" and 20.2" respectively. In the PAI technique, the bearings are calculated directly from adjusted coordinate of boundary marks. Based on the result, although there are nonexistence of bearing data in the

adjustment process, the calculated bearings remain accurate showing values that are closed to the original values from CP. In addition, systematic and gross errors of input bearing can be removed when the PAI technique is applied.

Table 5 shows the distance values from CP, DSMM and PAI. The NDCDB distances are the adjusted distances obtained from bearing adjustment method and PAI distances are the adjusted distances from angle adjustment method.

Table 5. Distance comparison

BM ID		DISTANCE(m)			DIFFERENCE(m)	
From	To	CP	DSMM	PAI	CP-DSMM	CP-PAI
5135308322	5147708142	21.793	21.793	21.79	0	0
5255108204	5236208200	18.907	18.918	18.92	-0.011	-0.009
5282507207	5299707376	24.156	24.166	24.18	-0.01	-0.023
5391408127	5403507966	20.115	20.136	20.12	-0.021	-0.009

The smallest difference of 0.000 m was found when there are no changes between the two methods. The largest differences for DSMM and PAI Technique are 0.021m and 0.023m respectively. The standard deviation of both method is 0.004m. From the overall data, 100% of both techniques are within the allowable tolerance of re-fixation which is not more than 0.050 m as stated in KPUP Circular Vol. 6/2009.

The PAI technique was applied to generate the high accuracy database. The generated cadastral database then has to be compared with the DSMM new dataset (NDCDB). Table 6 shows the result obtained from the adjustments.

Table 6. Coordinate and displacement

BM ID	NDCDB COORDINATE		PAI COORDINATE		DIFFERENCE		DISPLACEMENT
	EASTING	NORTHING	EASTING	NORTHING	ΔE	ΔN	
5219607465	15222.438	-60744.031	15222.440	-60743.998	-0.002	-0.033	0.033
5222507761	15225.294	-60773.583	15225.294	-60773.587	0.000	0.004	0.004
5280607603	15283.431	-60757.879	15283.442	-60757.869	-0.011	-0.010	0.015
5385507589	15388.305	-60756.402	15388.303	-60756.403	0.002	0.001	0.002
5252607863	15255.320	-60783.768	15255.320	-60783.768	0.000	0.000	0.000

For the total 200 points of the boundary marks, it shows that 100% are under 0.050 m having the lowest and highest values of 0.000 m and 0.033 m respectively as shown in 6. The standard deviation of coordinate difference is 0.009m. Based on the result, it indicates that PAI method yielded datasets that are within tolerance level compared to the NDCDB coordinate and at the same time the area and geometry preservation of original parcel is improved.

5. CONCLUSION

This study proposed an angle based LSA in the process of enhancing legacy dataset towards an accurate dataset. The proposed method is also capable of to reducing the systematic and gross errors obtained during bearing observation obtained from low accuracy datum. The interior angle data offer an independent raw data which is directly calculated from two bearing without take into account the quality of the observed bearing. The proposed technique demonstrated geometric fitting transformed efficiently in the PAI process due to the less standard deviation of area comparison between PAI and original CP parcel area. In addition, the adjusted distance, bearing and coordinate also fall within the reasonable tolerance compared to the original data in CP and new database NDCDB. Finally, this

study provides alternative tools for the enhancement of digital cadastral maps. These tools expectantly will assist the DSMM in managing the NDCDB toward implementing a more accurate digital cadastre in the next decade.

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